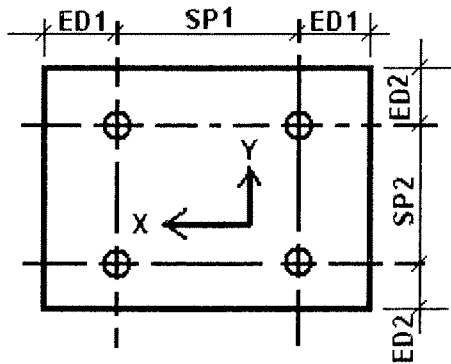


ANCHOR BOLT DESIGN (Canadian Code CSA A23.3 - 94):

ANCHOR BOLT DESIGN (Canadian Code CSA A23.3 - 94):



ANCHOR BOLT PLAN

Edge distance, $ED_1 = 310 \text{ mm}$ $ED_2 = 310 \text{ mm}$

Bolt Spacing, $SP_1 = 130 \text{ mm}$ $SP_2 = 130 \text{ mm}$

Bolt dia, $d_a = 25 \text{ mm}$ Total Bolt Length, $L_b = 750 \text{ mm}$

Bolt Projection: Projection = 150 mm

Embeeded Length $L_{emb} = L_b - \text{Projection} - 2 d_a$

$$L_{emb} = 550 \text{ mm}$$

Hexagonal nut dia. $d_{hex} = 1.5 d_a$ $d_{hex} = 37.5 \text{ mm}$

Compressive strength of concrete, $f_c = 30 \text{ MPa}$ Bolt area, $A_b = \frac{\pi}{4} d_a^2$ $A_b = 490.87 \text{ mm}^2$

Resistance factors for bolts, $\phi_{ar} = 0.67$ Tensile strength of anchor bolt, $F_{su_AB} = 414 \text{ MPa}$

Maximum Tension, $T_{fl} = 200 \text{ kN}$ Maximum Shear, $V_{fl} = 20 \text{ kN}$ (Column support reaction)

Tension on each Bolt, $T_f = \frac{T_{fl}}{4}$ $T_f = 50 \text{ kN}$ Maximum Shear, $V_f = \frac{V_{fl}}{4}$ $V_f = 5 \text{ kN}$

Dia of Vertical rebar, $\phi_{25} = 25 \text{ mm}$ Total no. $\phi_{no} = 16$ Area of 25 dia bar $A_{25} = 500 \text{ mm}^2$

Total area of Rebar, $A_{rebar} = \phi_{no} A_{25}$ $A_{rebar} = 8000 \text{ mm}^2$

Factor for reinforcing bar, $\phi_s = 0.85$ Yield strength of rebar, $f_y = 400 \text{ MPa}$ Concrete factor, $\phi_c = 0.60$

Edge Distance Check,

$$ED_{min} = \max \left(4 d_a, d_a \sqrt{\frac{F_{su_AB}}{6 f_c}} \right) \quad ED_{min} = 100 \text{ mm}$$

$ED_{check} = \text{if}(ED_{min} > \min(ED_1, ED_2), \text{"CHECK"}, \text{"OK"})$ $ED_{check} = \text{"OK"}$

ANCHOR BOLT DESIGN (Canadian Code CSA A23.3 - 94):

Bolt Strength Check,

Steel Strength Refer table 12.1 of Concrete Handbook, pg #12-9

Tensile capacity of each bolt, $T_1 = 0.5 \frac{\pi}{4} d_a^2 F_{su_AB}$ $T_1 = 101.61 \text{ kN}$

Bolt Check,
$$\text{Check1} = \begin{cases} \text{"Bolt Safe"} & \text{if } \left(T_f + \frac{V_f}{0.5} \right) \leq T_1 \\ \text{"CHECK"} & \text{otherwise} \end{cases}$$
 Check1 = "Bolt Safe"

Ductility Check,

Concrete Strength (Refer CSA A23.3-94, Appendix-D),

Effective stress area for tension in each bolt,

$A_{ten} = (ED_1 + 0.5 SP_1) (ED_2 + 0.5 SP_2)$ $A_{ten} = 1406.25 \text{ cm}^2$

Tensile capacity of each bolt, $T_2 = 0.30 \phi_c \sqrt{\frac{f_c}{\text{MPa}}} \text{ MPa } A_{ten}$ $T_2 = 138.64 \text{ kN}$

Check Ductility,
$$\text{Check2} = \begin{cases} \text{"DUCTILE"} & \text{if } T_2 \geq T_1 \\ \text{"Reinforcement reqd."} & \text{otherwise} \end{cases}$$
 Check2 = "DUCTILE"

$$\text{Ten} = \begin{cases} T_1 & \text{if Check2 = "Reinforcement reqd."} \\ 0 \text{ kN} & \text{otherwise} \end{cases}$$
 Ten = 0 kN

Area of rebar required for ductile failure, $A_{st} = \frac{\text{Ten}}{\phi_s f_y}$ A_{st} = 0 mm²

Development length of rebar depends on diameter of rebar.

Modified dia of rebar, $\phi_{mod} = \phi_{25} \frac{A_{st}}{0.25 A_{rebar}}$ ϕ_{mod} = 0 mm

Refer clause 12.2.3 of CSA - A23.3-94

For normal density concrete, $k_1 = 1.0$ $k_2 = 1.0$ $k_3 = 1.0$ $k_{4_20} = 1.0$ $k_4 = 0.8$

ANCHOR BOLT DESIGN (Canadian Code CSA A23.3 - 94):

Development length of bar, $L_d =$

$$0.45 k_1 k_2 k_3 k_4 \frac{20}{\sqrt{\frac{f_c}{\text{MPa}}}} \frac{f_y}{\text{MPa}} \phi_{\text{mod}} \quad \text{if } \phi_{\text{mod}} \leq 20 \text{ mm}$$

$$0.45 k_1 k_2 k_3 k_4 \frac{20}{\sqrt{\frac{f_c}{\text{MPa}}}} \frac{f_y}{\text{MPa}} \phi_{\text{mod}} \quad \text{if } \phi_{\text{mod}} > 20 \text{ mm}$$

$L_d = 0 \text{ mm}$

Development length available, $L_{d_avail} = L_{\text{emb}} - (\max(ED_1, ED_2) - 50\text{mm} - 25\text{mm})$ $L_{d_avail} = 315 \text{ mm}$

Checking of Development of Rebar, $\text{Check3} =$

$\text{"OK" if } L_{d_avail} \geq L_d$ $\text{"CHECK." otherwise}$
--

$\text{Check3} = \text{"OK"}$

Concrete Spalling check for shear force, Refer Figure D3, Appendix D, clause D4.3.6 (c)

Effective stress area for shear along X-X

$A_{vx} = (ED_1 + 0.5 SP_1) (2ED_2 + SP_2)$ $A_{vx} = 281250 \text{ mm}^2$

Effective stress area for shear along Y-Y,

$A_{vy} = (ED_2 + 0.5 SP_2) (2ED_1 + SP_1)$ $A_{vy} = 281250 \text{ mm}^2$

Effective Stress area for Shear, $A_v = \min(A_{vx}, A_{vy})$ $A_v = 281250 \text{ mm}^2$

Shear Capacity of concrete, $V_{\text{conc}} = 0.30 \phi_c \sqrt{\frac{f_c}{\text{MPa}}} \text{ MPa } A_v$ $V_{\text{conc}} = 277.28 \text{ kN}$

Checking of concrete spalling, $\text{Check4} =$

$\text{"OK" if } V_{\text{conc}} \geq V_{fl}$ $\text{"CHECK." otherwise}$
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$\text{Check4} = \text{"OK"}$